

Job NG2	Truss PS24	Truss Type SCISSORS	Qty 33	Ply 1	Job Reference (optional)
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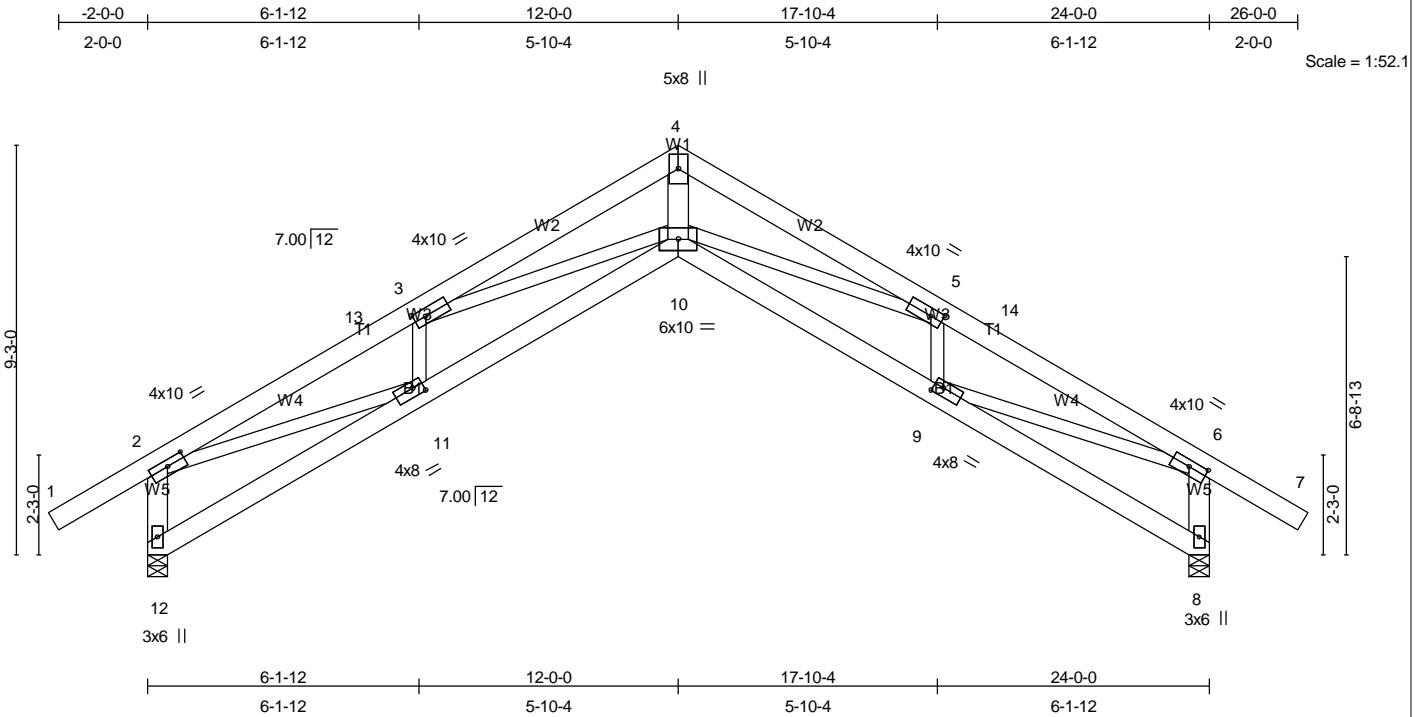


Plate Offsets (X,Y): [2:0-5-0,0-1-12], [3:0-3-4,0-2-0], [5:0-3-4,0-2-0], [6:0-5-0,0-1-12], [9:0-2-12,0-2-4], [11:0-2-12,0-2-4]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 85.0 (Roof Snow=85.0)	1-4-0 Plates Increase 1.00 Lumber Increase 1.00	TC 0.56 BC 0.53 WB 0.86 (Matrix)	in (loc) l/defl L/d Vert(LL) -0.45 10 >628 360 Vert(TL) -0.57 10 >494 240 Horz(TL) 0.73 8 n/a n/a Wind(LL) 0.07 10 >999 240	MT20	220/195
TCDL 7.0	Rep Stress Incr YES Code IBC2006/TPI2002			Weight: 177 lb	
BCLL 0.0					
BCDL 8.0					

**LUMBER**

TC 2 X 6 DF 1800F 1.6E  
 BC 2 X 6 DF 1800F 1.6E  
 W5 2 X 6 DF No.2, W5 2 X 6 DF No.2, W4 2 X 4 DF No.1&Btr, W1 2 X 6 DF No.2  
 WB 2 X 4 DF Stud/Std \*Except\* W4 2 X 4 DF No.1&Btr

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 3-6-0 oc purlins, except end verticals.  
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS**

(lb/size) 12=1857/0-5-8, 8=1857/0-5-8  
 Max Horz 12=-186(LC 5)  
 Max Uplift 12=-185(LC 7), 8=-185(LC 8)  
 Max Grav 12=1912(LC 2), 8=1912(LC 3)

**FORCES**

(lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 2-12=-1923/232, 1-2=0/279, 2-13=-4328/269, 3-13=-3982/284, 3-4=-5408/175, 4-5=-5332/186, 5-14=-3982/190, 6-14=-4327/182, 6-7=0/279, 6-8=-1923/184  
 BOT CHORD 11-12=-275/251, 10-11=-264/4140, 9-10=-106/4129, 8-9=-275/193  
 WEBS 2-11=-150/3503, 3-11=-1132/95, 3-10=0/1701, 4-10=-78/4496, 5-10=-185/1645, 5-9=-1111/74, 6-9=-91/3502

**NOTES**

- 1) Wind: ASCE 7-05; 85mph; h=25ft; TCDL=4.2psf; BCDL=4.8psf; Category II; Exp C; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) TCLL: ASCE 7-05; Pf=85.0 psf (flat roof snow); Category II; Exp C; Fully Exp.; Ct=1.1
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 17.0 psf or 2.00 times flat roof load of 85.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Bearing at joint(s) 12, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 185 lb uplift at joint 12 and 185 lb uplift at joint 8.
- 8) This truss is designed in accordance with the 2006 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

**LOAD CASE(S)** Standard